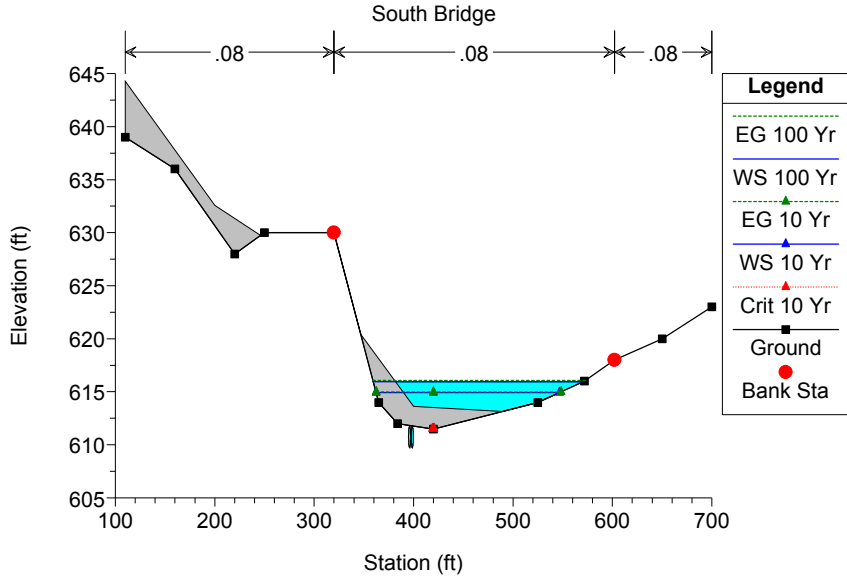
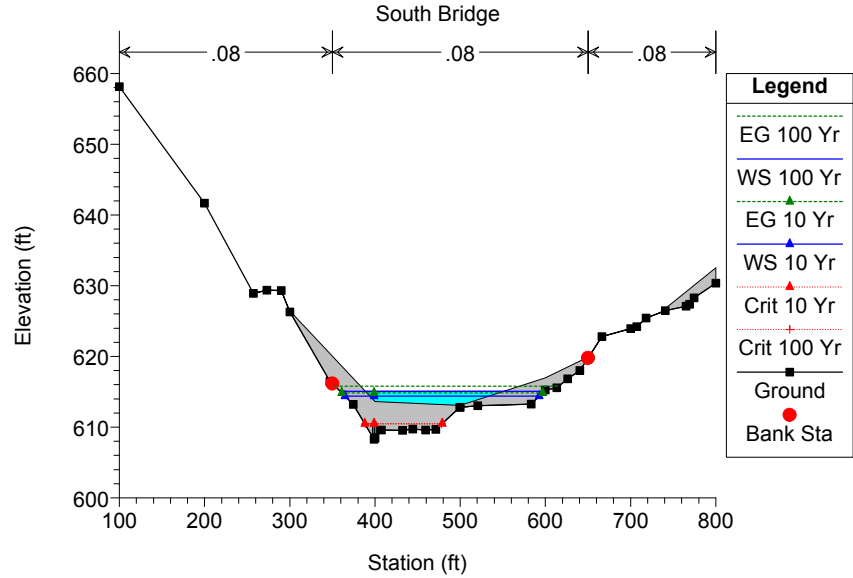


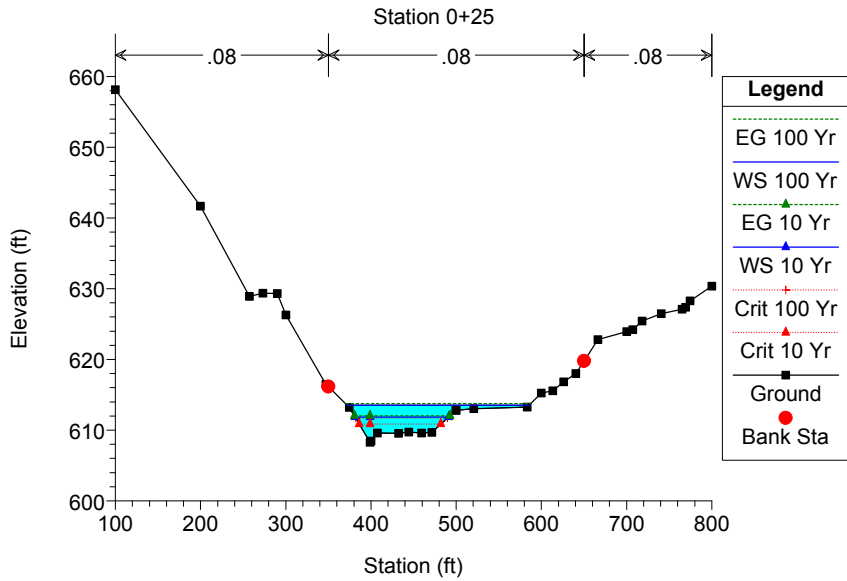
Valiano - Proposed Condition Plan: Plan 02 10/19/2015

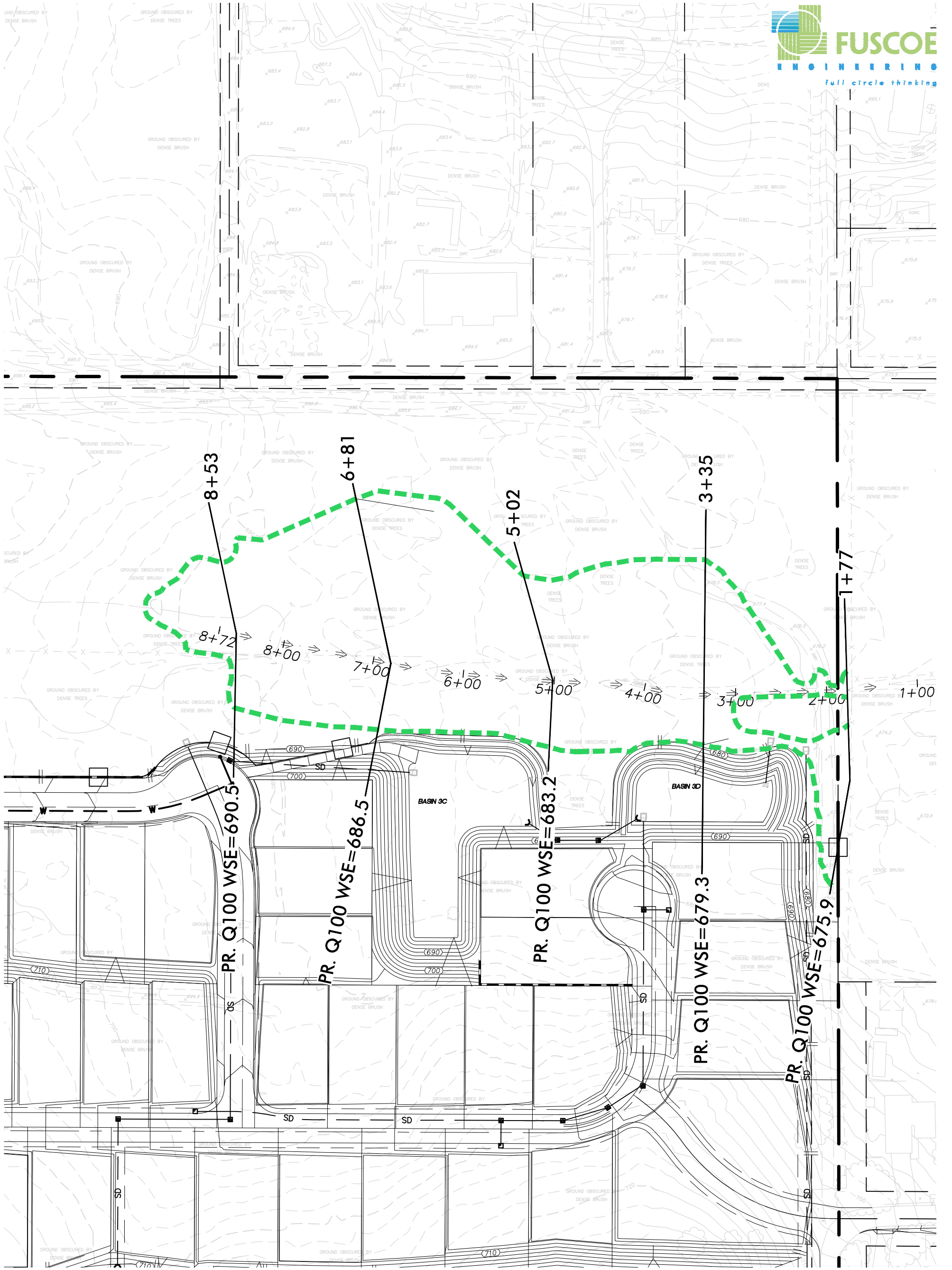


Valiano - Proposed Condition Plan: Plan 02 10/19/2015



Valiano - Proposed Condition Plan: Plan 02 10/19/2015

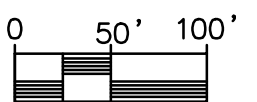




LEGEND

-  CHANNEL FLOW LINE
-  100 YEAR STORM INUNDATION LIMITS (PROPOSED CHANNEL, N = 0.08)

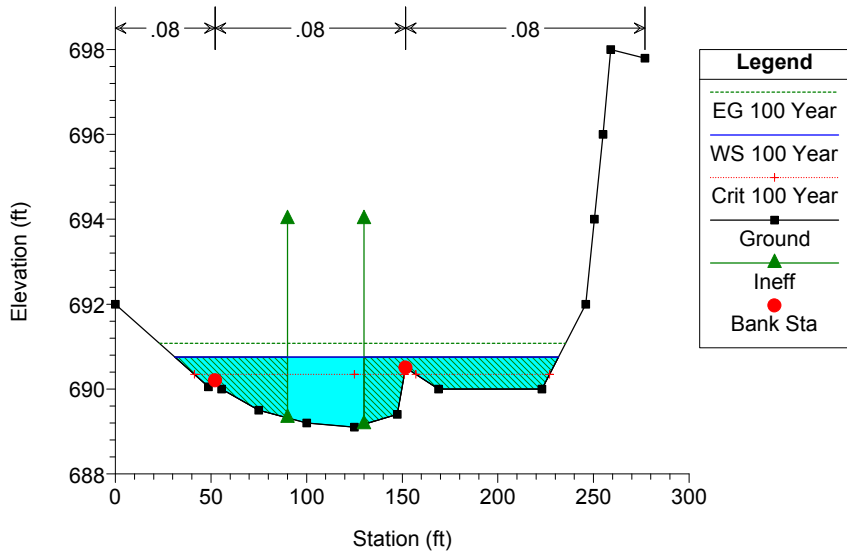
**VALIANO NEIGHBORHOOD 3
100-YEAR STORM INUNDATION EXHIBIT
SCALE: 1"=100'**



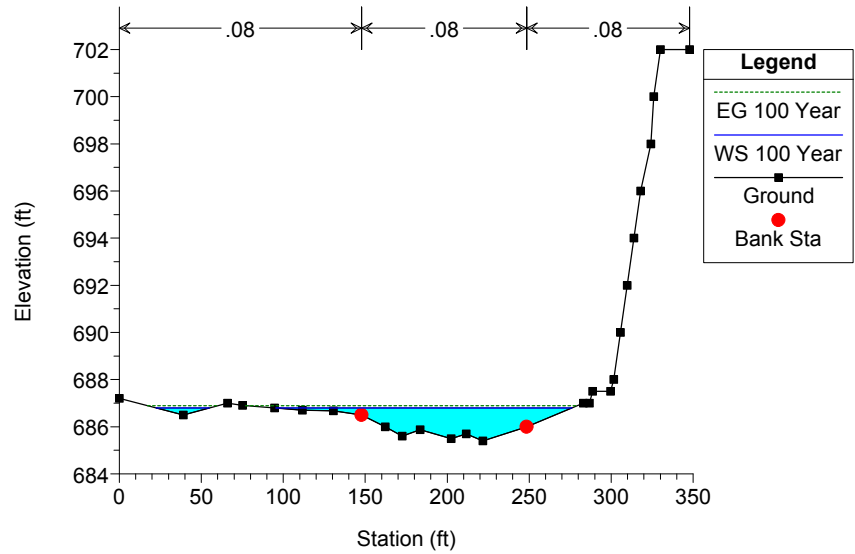
HEC-RAS Plan: Plan 01 River: Neighborhood 3 C Reach: Lower N3 Profile: 100 Year

Reach	River Sta	Profile	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
Lower N3	853	100 Year	287.00	689.10	690.75	690.34	691.07	0.032626	4.55	63.12	200.58	0.64
Lower N3	681	100 Year	287.00	685.40	686.80		686.89	0.018284	2.56	125.61	213.03	0.44
Lower N3	502	100 Year	287.00	681.98	683.25		683.35	0.021493	2.61	126.12	192.51	0.47
Lower N3	335	100 Year	287.00	677.88	679.39		679.50	0.024778	3.40	122.62	199.30	0.53
Lower N3	177	100 Year	287.00	674.20	675.92	675.59	675.99	0.020037	2.17	140.90	206.61	0.44

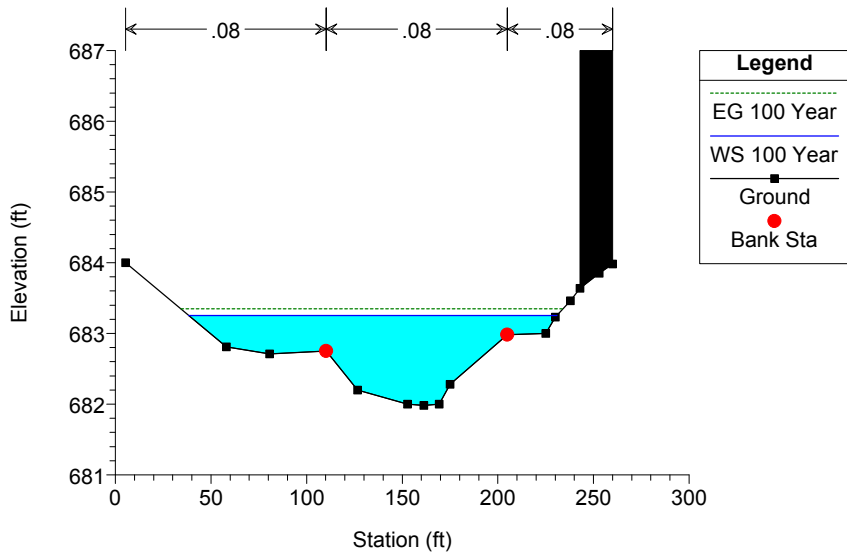
Neighborhood 3_1 Plan: Plan 01 10/16/2015



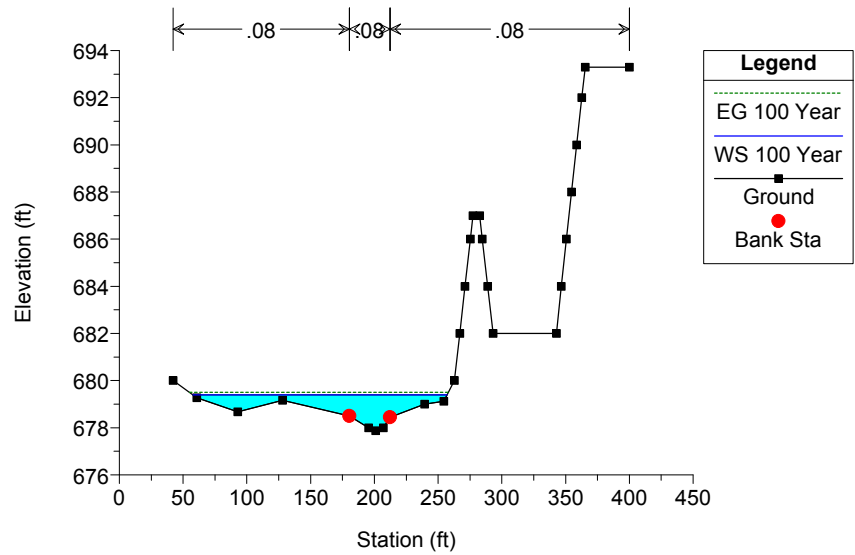
Neighborhood 3_1 Plan: Plan 01 10/16/2015



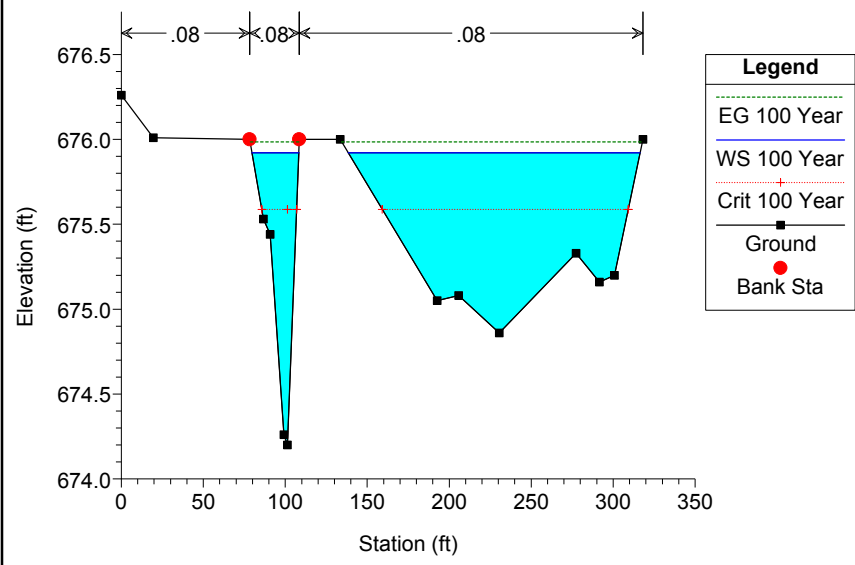
Neighborhood 3_1 Plan: Plan 01 10/16/2015



Neighborhood 3_1 Plan: Plan 01 10/16/2015



Neighborhood 3_1 Plan: Plan 01 10/16/2015



Appendix G

Hill Valley Road Analysis

San Dieguito Community Plan Area – C Value Reference

Hill Valley Road Basin Coding Summary

Hill Valley Road AES Output

Existing Conditions Culvert Analysis

Proposed Conditions Culvert Analysis

Hill Valley Road Crossing Exhibit

San Dieguito Community Planning Area

General Plan Land Use Designations^{1,2}
Adopted August 2011

- Village Residential (VR-30)
- Village Residential (VR-24)
- Village Residential (VR-20)
- Village Residential (VR-15)
- Village Residential (VR-10.9)
- Village Residential (VR-7.3)
- Village Residential (VR-4.3)
- Village Residential (VR-2.9)
- Village Residential (VR-2)
- Semi-Rural Residential (SR-5)
- Semi-Rural Residential (SR-1)
- Semi-Rural Residential (SR-2)
- Semi-Rural Residential (SR-4)
- Semi-Rural Residential (SR-10)
- Rural Lands (RL-20)
- Rural Lands (RL-40)
- Rural Lands (RL-80)
- Specific Plan Area (residential densities in italics)⁴
- Office Professional³
- Neighborhood Commercial³
- General Commercial³
- Rural Commercial³
- Limited Impact Industrial³
- Medium Impact Industrial³
- High Impact Industrial³
- Village Core Mixed Use
- Public/Semi-Public Facilities³
- Public/Semi-Public Lands (Solid Waste Facility)
- Public Agency Lands
- Tribal Lands
- Open Space (Recreation)
- Open Space (Conservation)
- County Water Authority Boundary
- San Dieguito Community Planning Area Boundary

NOTES:

- 1: The type and intensity of development depicted on the map must be implemented in accordance with General Plan goals and policies and other County regulations which may further affect the type and intensity of use.
- 2: Land Use Element, Table LU-1 indicates the applicable Regional Category for each designation.
- 3: Maximum development intensity for non-residential designations is provided in Land Use Element, Table LU-1.
- 4: Refer to Community Plan for general land uses and intensities allowed in Specific Plan Area (SPA).

Map Prepared By:



Source: County of San Diego, SANDAG

Source: County of San Diego, SANDAG

Source: County of San Diego, SANDAG

Source: County of San Diego, SANDAG

Source: County of San Diego, SANDAG

Source: County of San Diego, SANDAG

Source: County of San Diego, SANDAG

Source: County of San Diego, SANDAG

Source: County of San Diego, SANDAG

Source: County of San Diego, SANDAG

Source: County of San Diego, SANDAG

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Source: County of San Diego, SANDAG

Source: County of San Diego, SANDAG

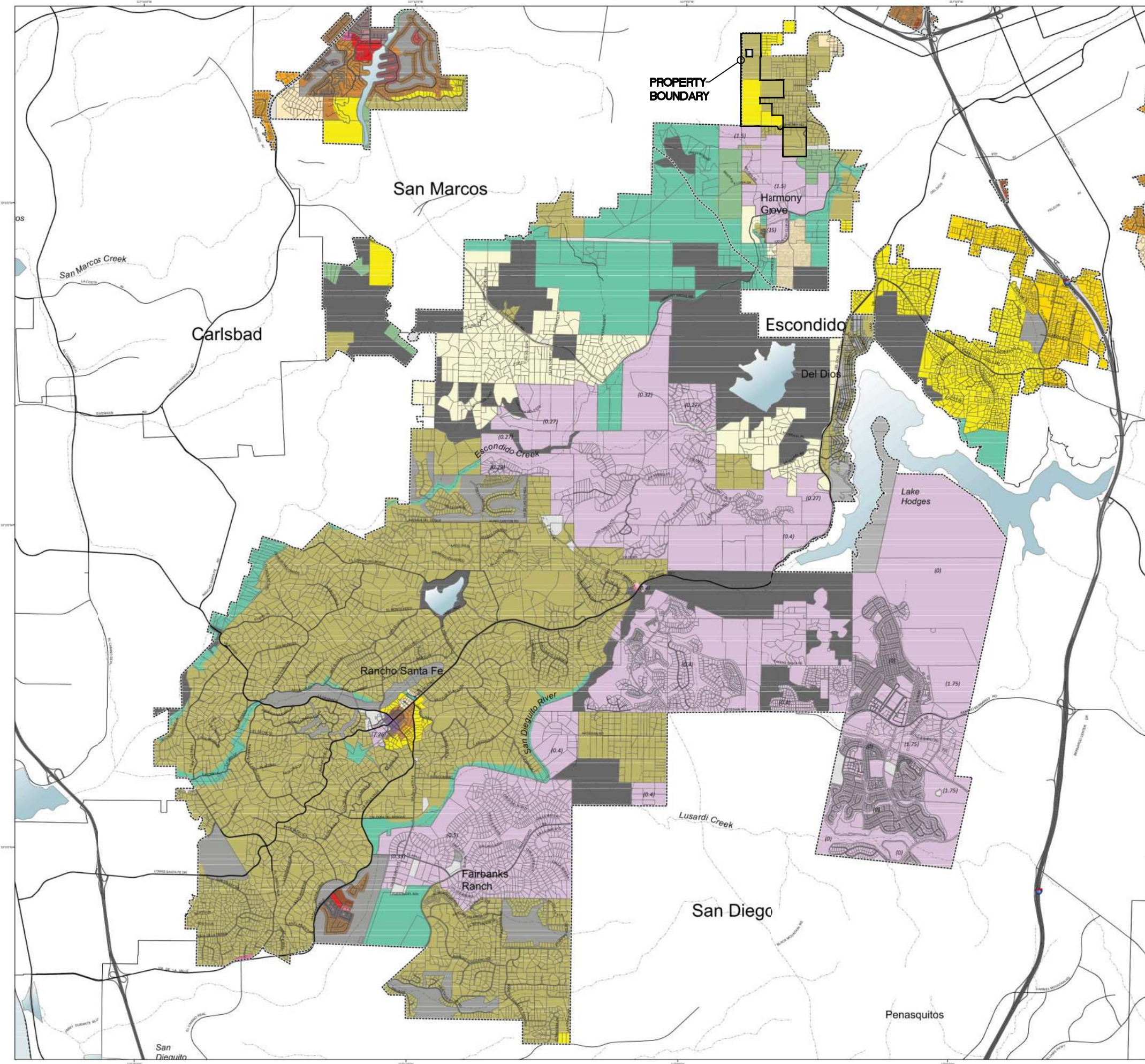
Source: County of San Diego, SANDAG

Source: County of San Diego, SANDAG

Source: County of San Diego, SANDAG



Printed: July 23, 2012



RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT
2003, 1985, 1981 HYDROLOGY MANUAL
(c) Copyright 1982-2010 Advanced Engineering Software (aes)
Ver. 17.0 Release Date: 07/01/2010 License ID 1355

Analysis prepared by:

Fusco Engineering, Inc.
6390 Greenwich Drive, Suite 170
San Diego, CA 92122

***** DESCRIPTION OF STUDY *****

* VALIANO *
* HILL VALLEY ROAD CROSSING HYDROLOGY *
* JULY 5, 2013 *

FILE NAME: HVR.DAT
TIME/DATE OF STUDY: 11:31 07/05/2013

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

2003 SAN DIEGO MANUAL CRITERIA

USER SPECIFIED STORM EVENT(YEAR) = 100.00
6-HOUR DURATION PRECIPITATION (INCHES) = 3.000
SPECIFIED MINIMUM PIPE SIZE(INCH) = 12.00

SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD

NOTE: USE MODIFIED RATIONAL METHOD PROCEDURES FOR CONFLUENCE ANALYSIS

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

Table with columns: NO., WIDTH (FT), CROWN TO CROSSFALL (FT), STREET-CROSSFALL: IN- / OUT- / PARK- SIDE / SIDE / WAY, CURB HEIGHT (FT), GUTTER WIDTH (FT), GUTTER LIP (FT), GUTTER GEOMETRIES: HIKE (FT), MANNING FACTOR (n). Row 1: 1, 30.0, 20.0, 0.018/0.018/0.020, 0.67, 2.00, 0.0313, 0.167, 0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

- 1. Relative Flow-Depth = 0.00 FEET as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

SIZE PIPE WITH A FLOW CAPACITY GREATER THAN OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.

FLOW PROCESS FROM NODE 1.00 TO NODE 2.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====

RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .3600
SOIL CLASSIFICATION IS "C"
S.C.S. CURVE NUMBER (AMC II) = 76
INITIAL SUBAREA FLOW-LENGTH(FEET) = 100.00
UPSTREAM ELEVATION(FEET) = 808.00
DOWNSTREAM ELEVATION(FEET) = 801.00
ELEVATION DIFFERENCE(FEET) = 7.00
SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.964
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 6.384

HVR. TXT

SUBAREA RUNOFF(CFS) = 0.90
TOTAL AREA(ACRES) = 0.39 TOTAL RUNOFF(CFS) = 0.90

FLOW PROCESS FROM NODE 2.00 TO NODE 3.00 IS CODE = 53

>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	801.00	DOWNSTREAM(FEET) =	785.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	215.00	CHANNEL SLOPE =	0.0744

SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .0744 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION
CHANNEL FLOW THRU SUBAREA(CFS) = 0.90
FLOW VELOCITY(FEET/SEC) = 1.53 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 2.35 Tc(MIN.) = 9.31
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 3.00 = 315.00 FEET.

FLOW PROCESS FROM NODE 2.00 TO NODE 3.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	5.294		
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT =	.3600		
SOIL CLASSIFICATION IS "C"			
S. C. S. CURVE NUMBER (AMC II) =	76		
AREA-AVERAGE RUNOFF COEFFICIENT =	0.3600		
SUBAREA AREA(ACRES) =	1.78	SUBAREA RUNOFF(CFS) =	3.39
TOTAL AREA(ACRES) =	2.2	TOTAL RUNOFF(CFS) =	4.14
TC(MIN.) =	9.31		

FLOW PROCESS FROM NODE 3.00 TO NODE 4.00 IS CODE = 53

>>>>COMPUTE NATURAL MOUNTAIN CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	785.00	DOWNSTREAM(FEET) =	724.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	812.00	CHANNEL SLOPE =	0.0751

SLOPE ADJUSTMENT CURVE USED:
EFFECTIVE SLOPE = .0751 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
CHANNEL FLOW THRU SUBAREA(CFS) = 4.14
FLOW VELOCITY(FEET/SEC) = 2.46 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 5.50 Tc(MIN.) = 14.81
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4.00 = 1127.00 FEET.

FLOW PROCESS FROM NODE 3.00 TO NODE 4.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	3.924		
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT =	.3600		
SOIL CLASSIFICATION IS "C"			
S. C. S. CURVE NUMBER (AMC II) =	76		
AREA-AVERAGE RUNOFF COEFFICIENT =	0.3600		
SUBAREA AREA(ACRES) =	13.05	SUBAREA RUNOFF(CFS) =	18.44
TOTAL AREA(ACRES) =	15.2	TOTAL RUNOFF(CFS) =	21.50
TC(MIN.) =	14.81		

HVR.TXT

FLOW PROCESS FROM NODE 4.00 TO NODE 5.00 IS CODE = 52

>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	724.00	DOWNSTREAM(FEET) =	702.00
CHANNEL LENGTH THRU SUBAREA(FEET) =	812.00	CHANNEL SLOPE =	0.0271
CHANNEL FLOW THRU SUBAREA(CFS) =	21.50		
FLOW VELOCITY(FEET/SEC) =	5.03	(PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)	
TRAVEL TIME(MIN.) =	2.69	Tc(MIN.) =	17.50
LONGEST FLOWPATH FROM NODE	1.00 TO NODE	5.00 =	1939.00 FEET.

FLOW PROCESS FROM NODE 4.00 TO NODE 5.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) =	3.524		
RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT =	.3600		
SOIL CLASSIFICATION IS "C"			
S.C.S. CURVE NUMBER (AMC II) =	76		
AREA-AVERAGE RUNOFF COEFFICIENT =	0.3600		
SUBAREA AREA(ACRES) =	32.92	SUBAREA RUNOFF(CFS) =	41.76
TOTAL AREA(ACRES) =	48.1	TOTAL RUNOFF(CFS) =	61.07
TC(MIN.) =	17.50		

=====

END OF STUDY SUMMARY:

TOTAL AREA(ACRES)	=	48.1	TC(MIN.) =	17.50
PEAK FLOW RATE(CFS)	=	61.07		

=====

END OF RATIONAL METHOD ANALYSIS

♀

Existing Condition
HY-8 Culvert Analysis Report

Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 61 cfs

Design Flow: 61 cfs

Maximum Flow: 61 cfs

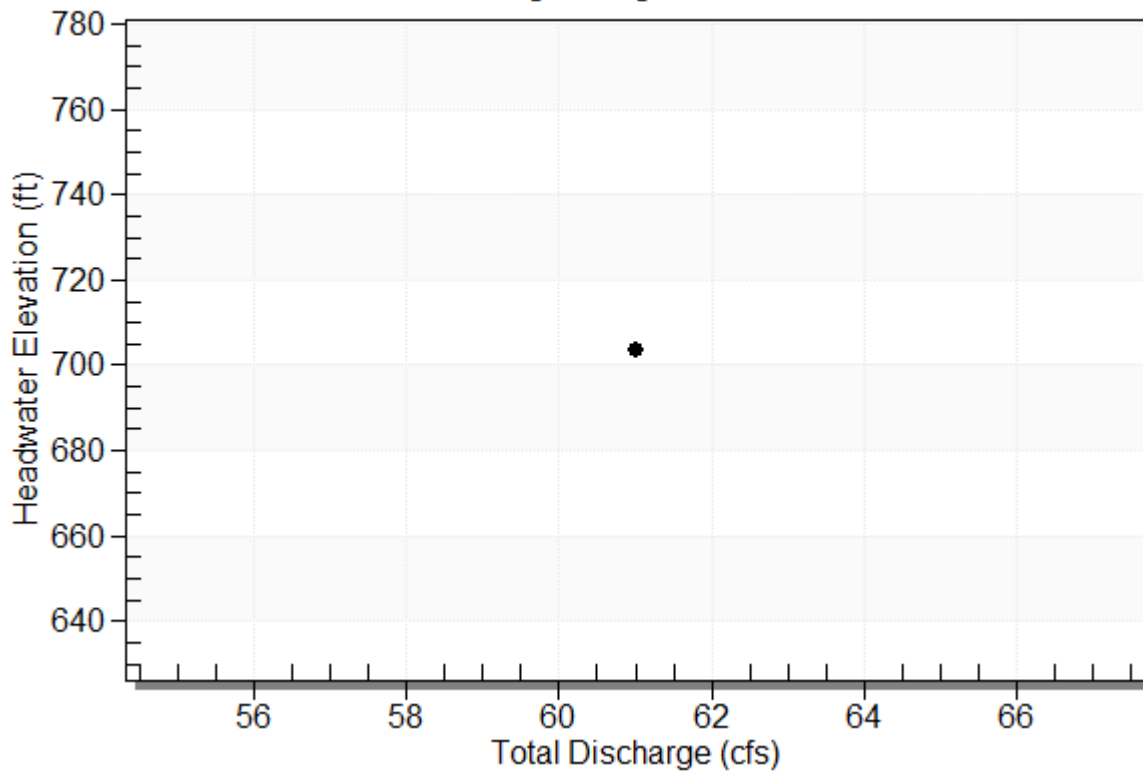
Table 1 - Summary of Culvert Flows at Crossing: Existing Conditions

Headwater Elevation (ft)	Total Discharge (cfs)	Existing 24 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
703.75	61.00	24.57	36.41	8
703.75	61.00	24.57	36.41	2
703.75	61.00	24.57	36.41	2
703.75	61.00	24.57	36.41	2
703.75	61.00	24.57	36.41	2
703.75	61.00	24.57	36.41	2
703.75	61.00	24.57	36.41	2
703.75	61.00	24.57	36.41	2
703.75	61.00	24.57	36.41	2
703.75	61.00	24.57	36.41	2
703.75	61.00	24.57	36.41	2
703.75	61.00	24.57	36.41	2
703.50	23.28	23.28	0.00	Overtopping

Rating Curve Plot for Crossing: Existing Conditions

Total Rating Curve

Crossing: Existing Conditions



Straight Culvert

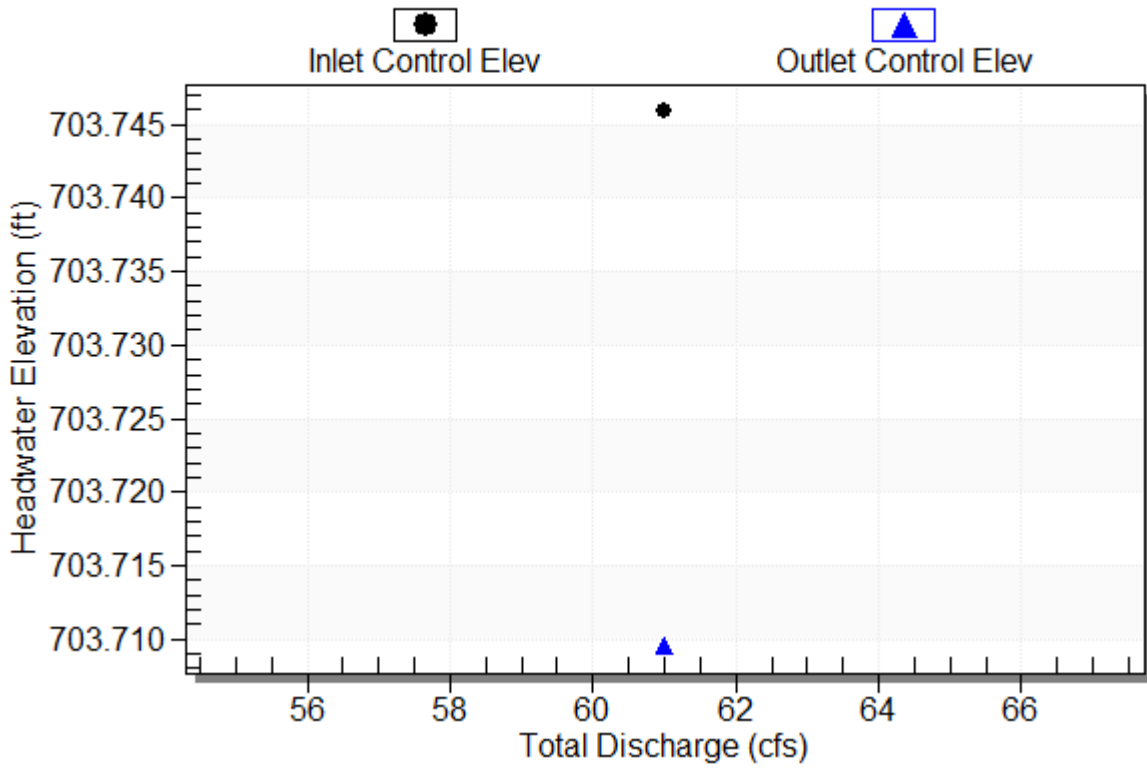
Inlet Elevation (invert): 700.00 ft, Outlet Elevation (invert): 699.70 ft

Culvert Length: 60.00 ft, Culvert Slope: 0.0050

Culvert Performance Curve Plot: Existing 24

Performance Curve

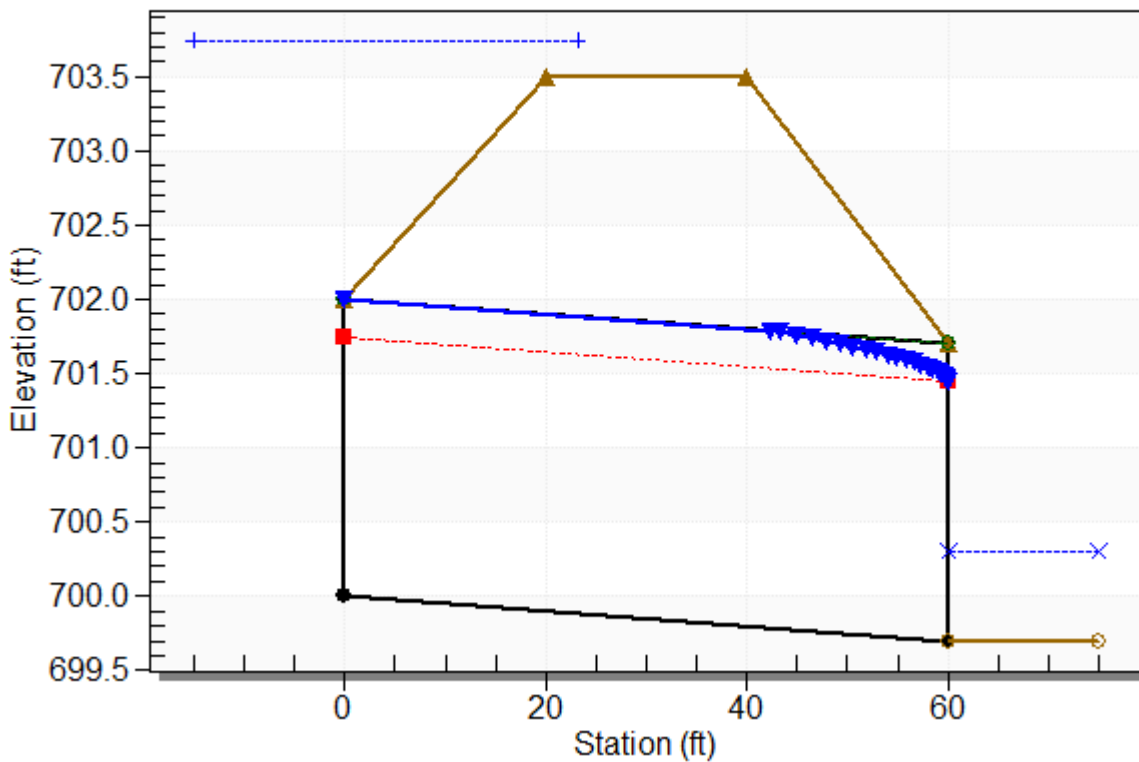
Culvert: Existing 24



Water Surface Profile Plot for Culvert: Existing 24

Crossing - Existing Conditions, Design Discharge - 61.0 cfs

Culvert - Existing 24, Culvert Discharge - 24.6 cfs



Site Data - Existing 24

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft

Inlet Elevation: 700.00 ft

Outlet Station: 60.00 ft

Outlet Elevation: 699.70 ft

Number of Barrels: 1

Culvert Data Summary - Existing 24

Barrel Shape: Circular

Barrel Diameter: 2.00 ft

Barrel Material: Concrete

Embedment: 0.00 in

Barrel Manning's n: 0.0130

Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall

Inlet Depression: NONE

Tailwater Channel Data - Existing Conditions

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 12.00 ft

Side Slope (H:V): 25.00 (1:1)

Channel Slope: 0.0200

Channel Manning's n: 0.0300

Channel Invert Elevation: 699.70 ft

Roadway Data for Crossing: Existing Conditions

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 100.00 ft

Crest Elevation: 703.50 ft

Roadway Surface: Paved

Roadway Top Width: 20.00 ft

Proposed Condition

HY-8 Culvert Analysis Report

Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 61 cfs

Design Flow: 61 cfs

Maximum Flow: 61 cfs

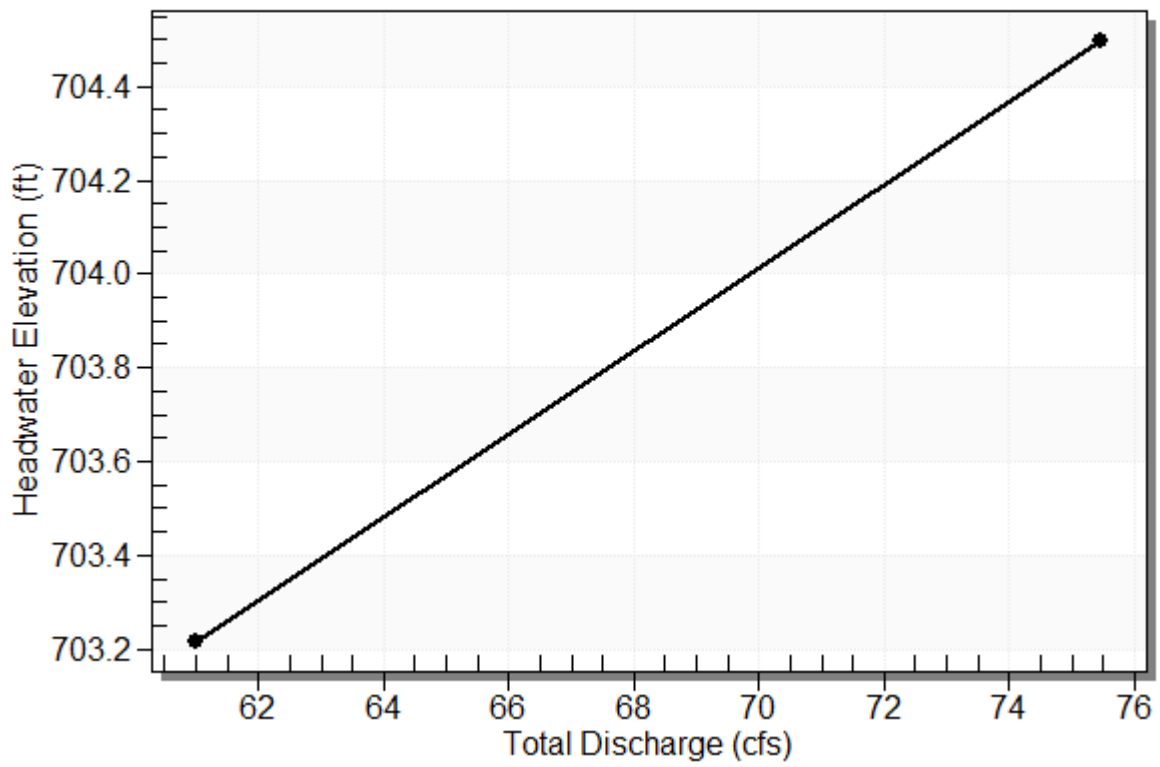
Table 1 - Summary of Culvert Flows at Crossing: Proposed Conditions

Headwater Elevation (ft)	Total Discharge (cfs)	Proposed Dual 30 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
703.22	61.00	61.00	0.00	1
703.22	61.00	61.00	0.00	1
703.22	61.00	61.00	0.00	1
703.22	61.00	61.00	0.00	1
703.22	61.00	61.00	0.00	1
703.22	61.00	61.00	0.00	1
703.22	61.00	61.00	0.00	1
703.22	61.00	61.00	0.00	1
703.22	61.00	61.00	0.00	1
703.22	61.00	61.00	0.00	1
703.22	61.00	61.00	0.00	1
704.00	75.46	75.46	0.00	Overtopping

Rating Curve Plot for Crossing: Proposed Conditions

Total Rating Curve

Crossing: Proposed Conditions



Straight Culvert

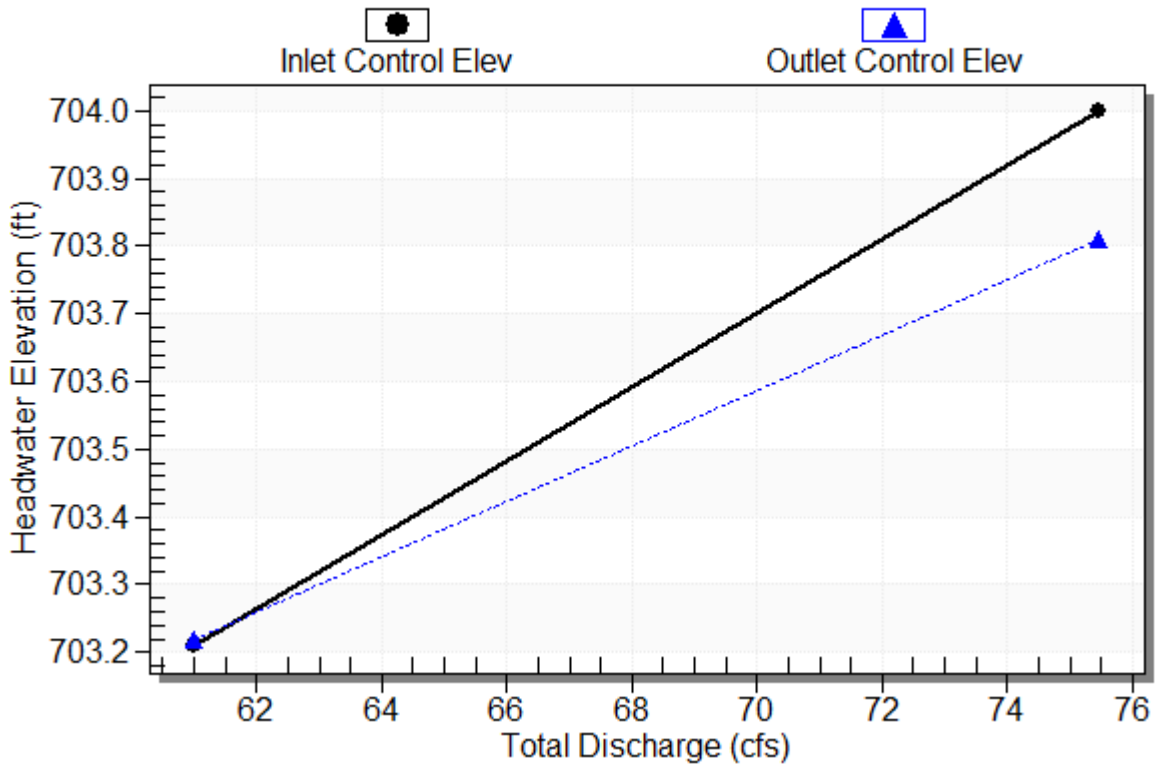
Inlet Elevation (invert): 700.00 ft, Outlet Elevation (invert): 699.80 ft

Culvert Length: 40.00 ft, Culvert Slope: 0.0050

Culvert Performance Curve Plot: Proposed Dual 30

Performance Curve

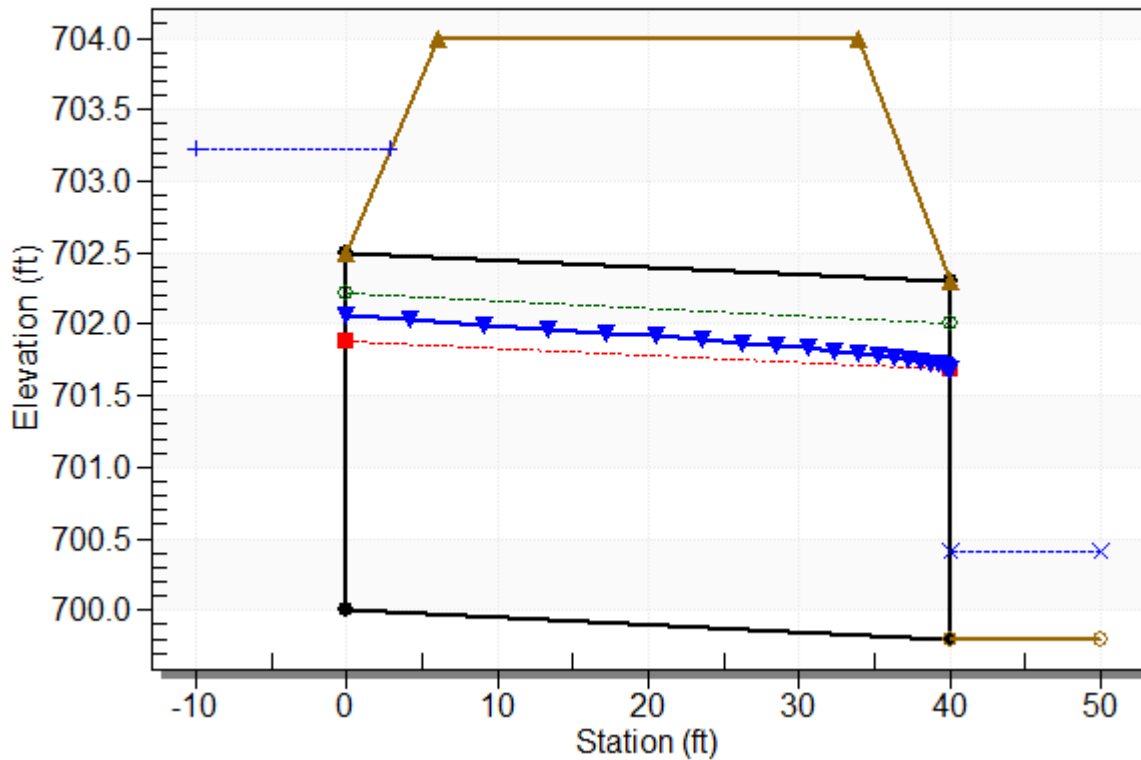
Culvert: Proposed Dual 30



Water Surface Profile Plot for Culvert: Proposed Dual 30

Crossing - Proposed Conditions, Design Discharge - 61.0 cfs

Culvert - Proposed Dual 30, Culvert Discharge - 61.0 cfs



Site Data - Proposed Dual 30

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft

Inlet Elevation: 700.00 ft

Outlet Station: 40.00 ft

Outlet Elevation: 699.80 ft

Number of Barrels: 2

Culvert Data Summary - Proposed Dual 30

Barrel Shape: Circular

Barrel Diameter: 2.50 ft

Barrel Material: Concrete

Embedment: 0.00 in

Barrel Manning's n: 0.0130

Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall

Inlet Depression: NONE

Tailwater Channel Data - Proposed Conditions

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 12.00 ft

Side Slope (H:V): 25.00 (1:1)

Channel Slope: 0.0200

Channel Manning's n: 0.0300

Channel Invert Elevation: 699.80 ft

Roadway Data for Crossing: Proposed Conditions

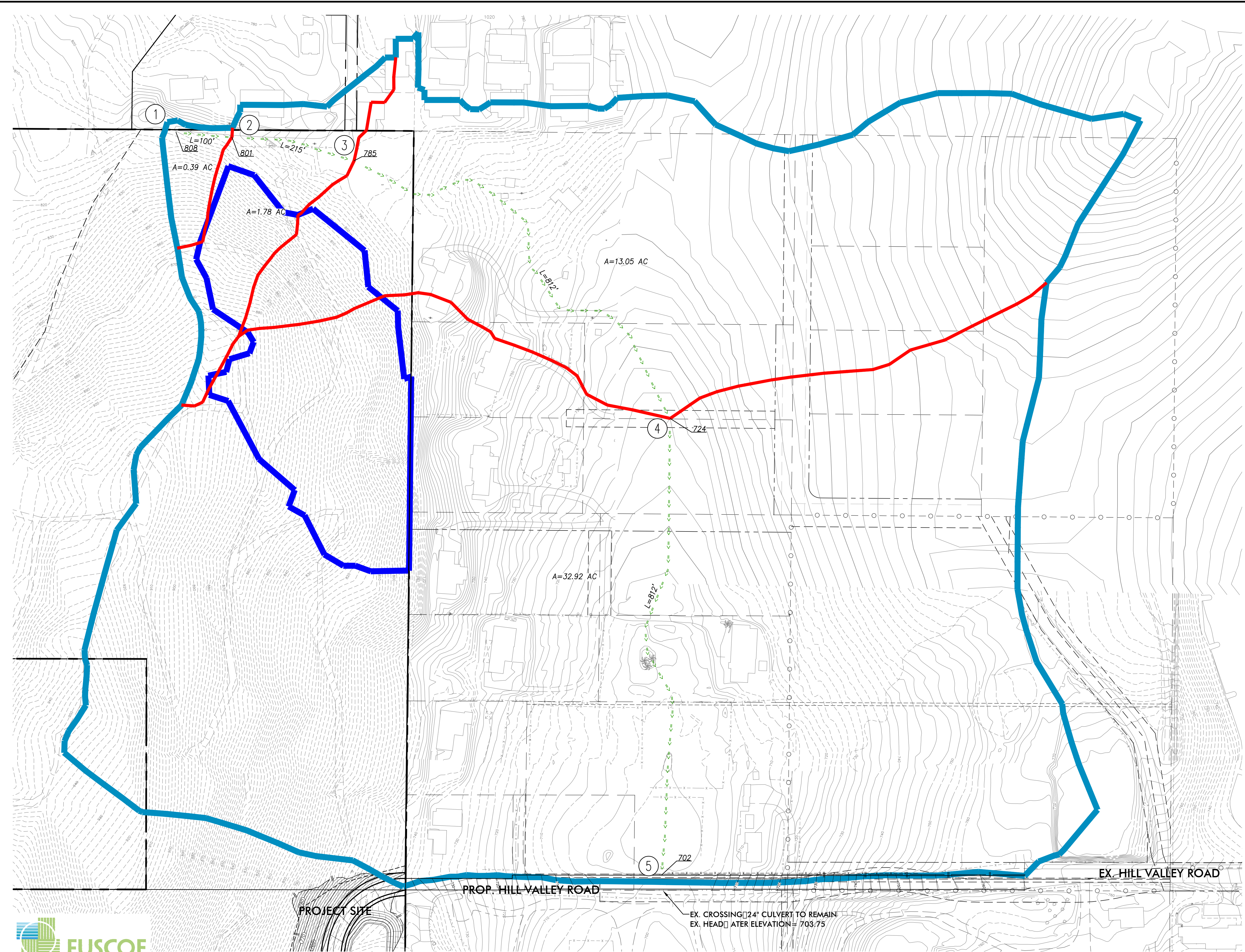
Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 100.00 ft

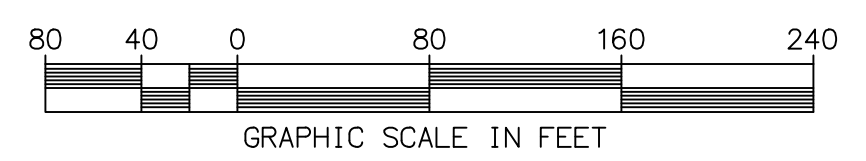
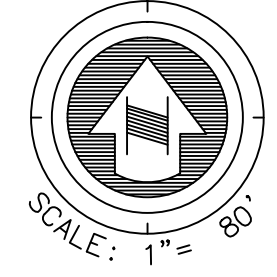
Crest Elevation: 704.00 ft

Roadway Surface: Paved

Roadway Top Width: 28.00 ft



LEGEND	
PROPERTY LINE	---
PROPOSED ACCESS EASEMENT	- - - -
EXISTING CONTOUR	~ ~ ~ ~
PROPOSED CONTOUR	~ ~ ~ ~
BASIN BOUNDARY	— (Blue)
SUB-BASIN BOUNDARY	— (Red)
FLOW PATH	- - - -> (Green)



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 www.fuscoe.com

VALIANO
HILL VALLEY ROAD CROSSING
MARCH 2015

VA-PROJECTS\02090 - INTEGRAL COMMUNITIES\07-02-EDEN HILLS ENGINEERING\ENGINEERING\HILL VALLEY ROAD CROSSING.DWG (03-10-15 2: Printed by: Erika Gentry)

Appendix H

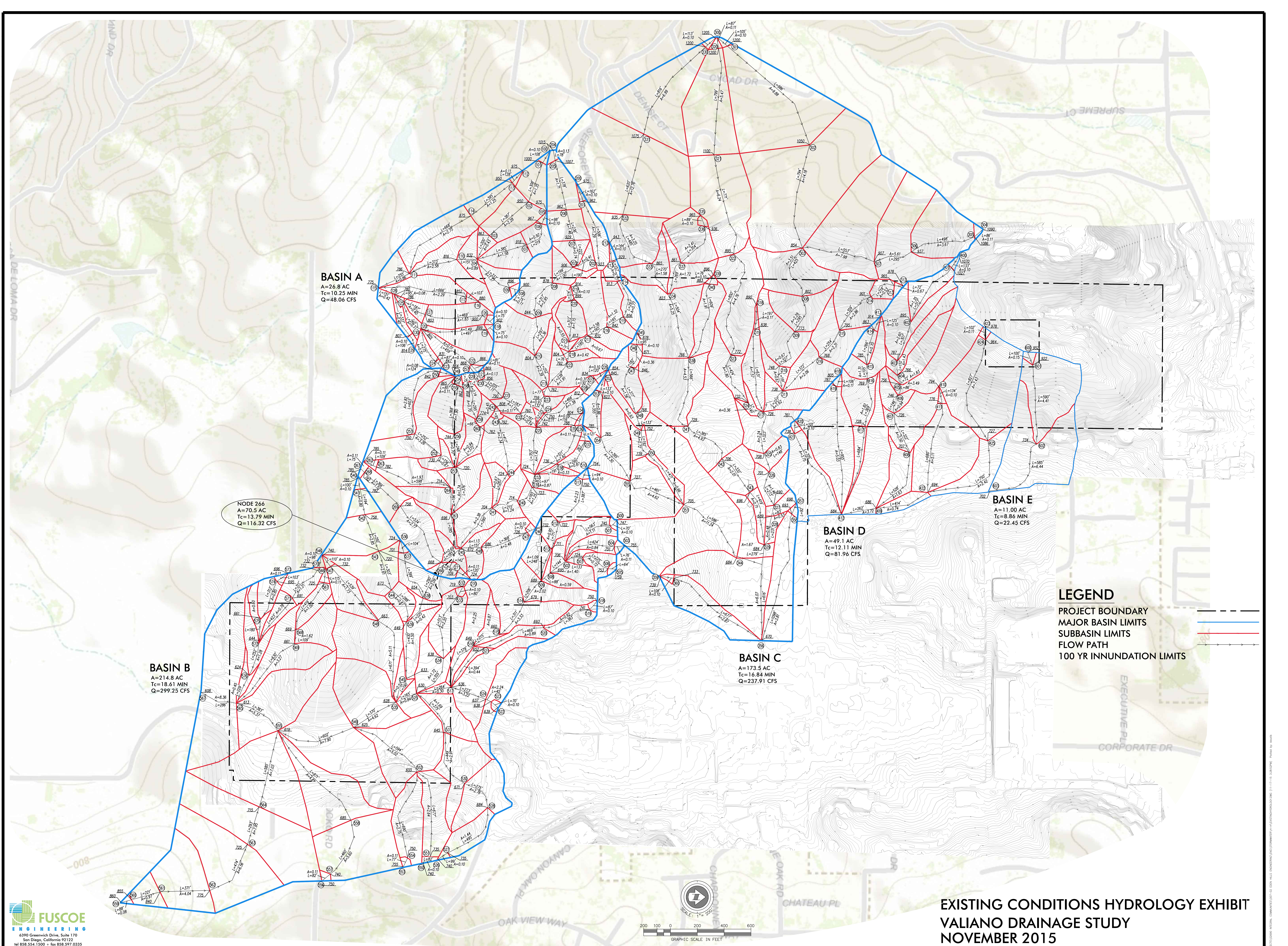
Map Pocket

Existing Conditions Hydrology Exhibit

Proposed Conditions Hydrology Exhibit

Valiano Neighborhood 5 100 Year Storm Inundation Exhibit

Valiano Neighborhood 3 100 Year Storm Inundation Exhibit



BASIN A
 A=26.8 AC
 Tc=10.25 MIN
 Q=48.06 CFS

NODE 266
 A=70.5 AC
 Tc=13.79 MIN
 Q=116.32 CFS

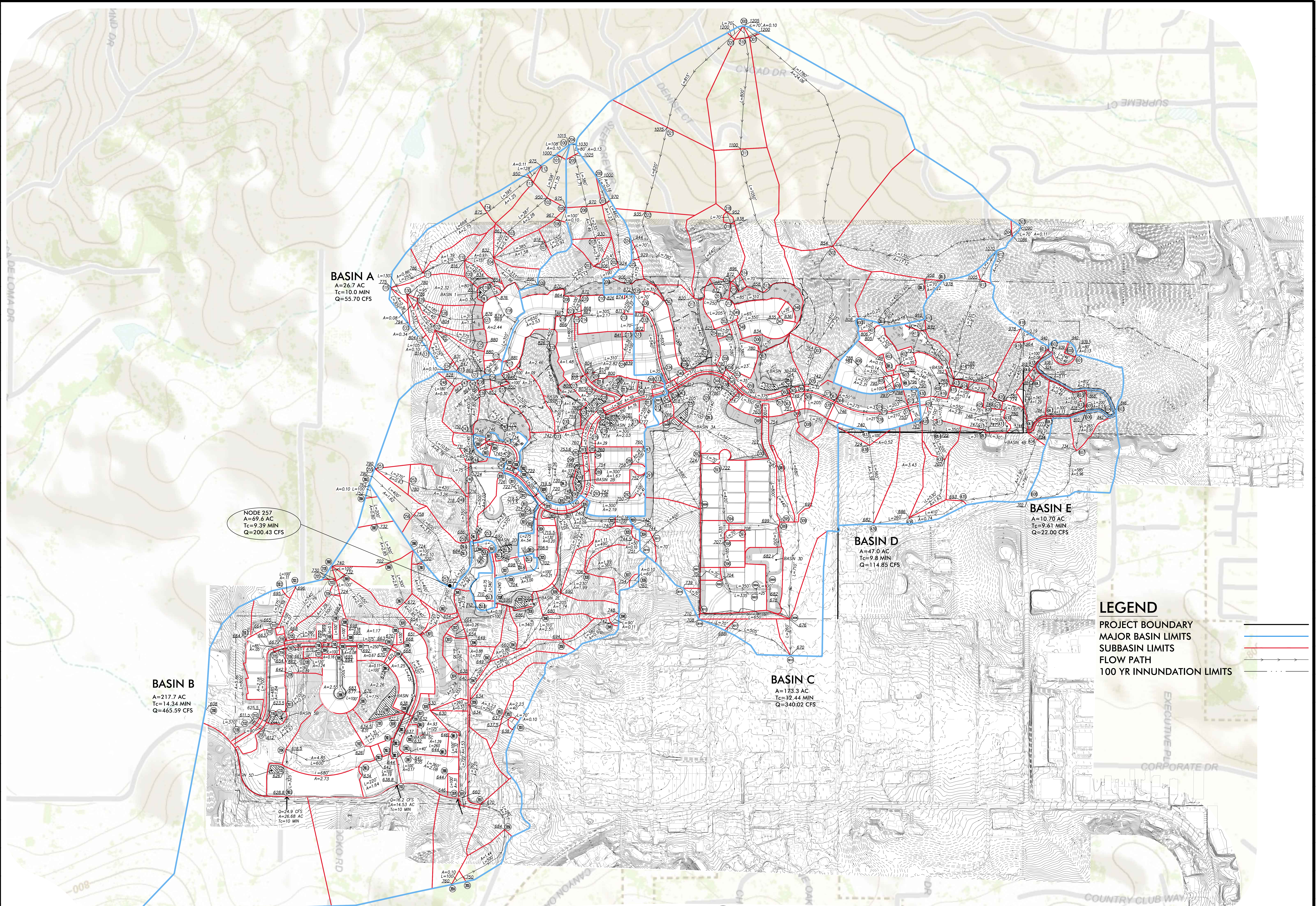
BASIN E
 A=11.00 AC
 Tc=8.86 MIN
 Q=22.45 CFS

BASIN D
 A=49.1 AC
 Tc=12.11 MIN
 Q=81.96 CFS

BASIN C
 A=173.5 AC
 Tc=16.84 MIN
 Q=237.91 CFS

BASIN B
 A=214.8 AC
 Tc=18.61 MIN
 Q=299.25 CFS

LEGEND
 PROJECT BOUNDARY ———
 MAJOR BASIN LIMITS ———
 SUBBASIN LIMITS ———
 FLOW PATH ———
 100 YR INNUNDATION LIMITS - - - - -



BASIN A
 A=26.7 AC
 Tc=10.0 MIN
 Q=55.70 CFS

NODE 257
 A=69.6 AC
 Tc=9.39 MIN
 Q=200.43 CFS

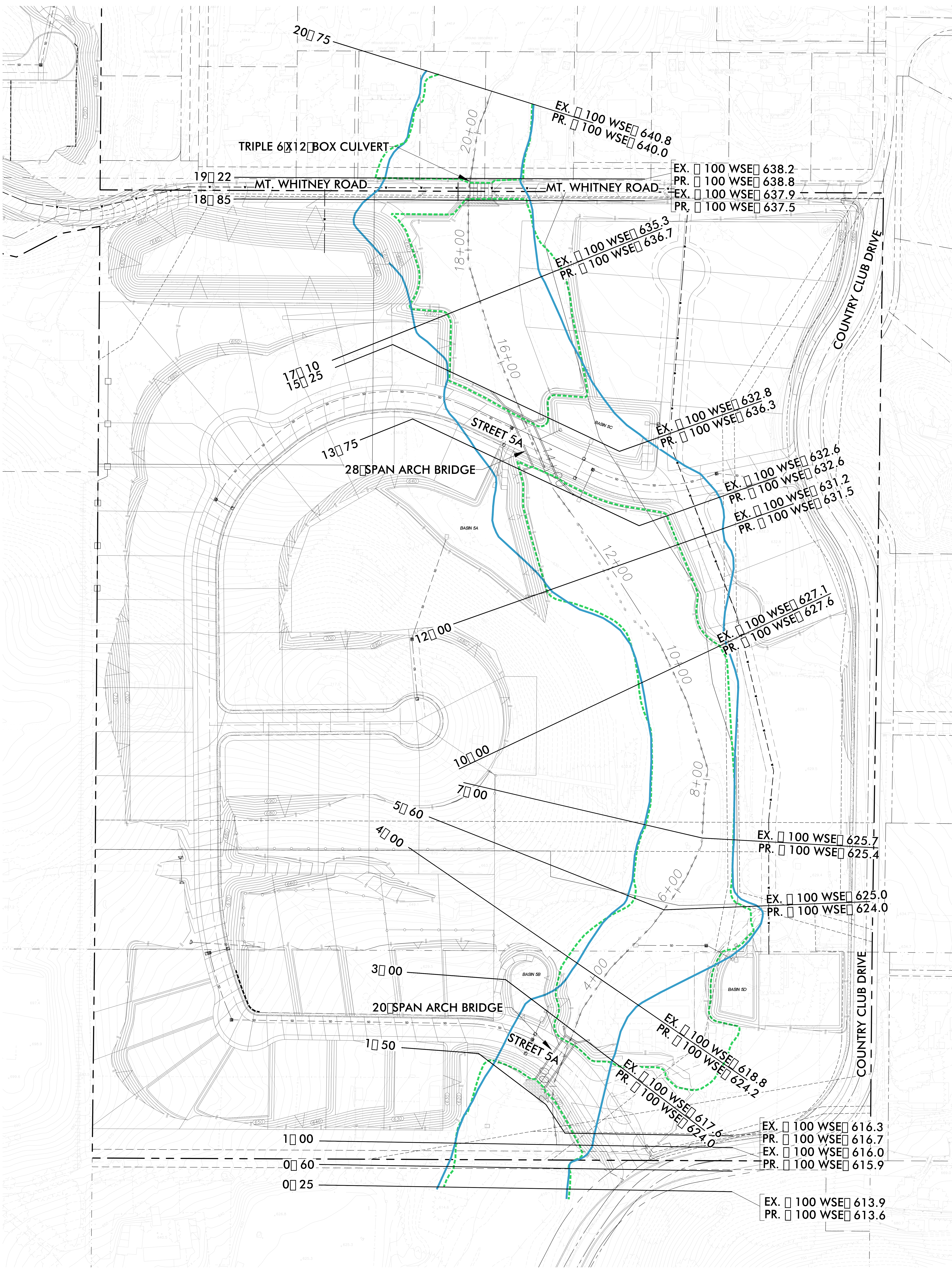
BASIN B
 A=217.7 AC
 Tc=14.34 MIN
 Q=465.59 CFS

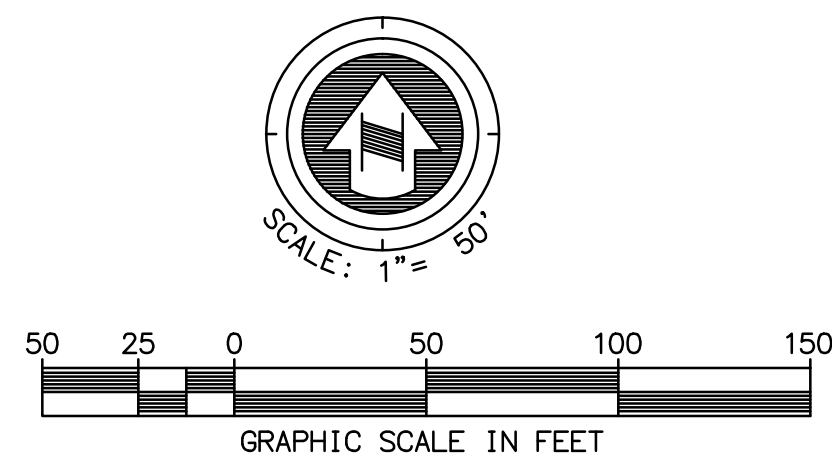
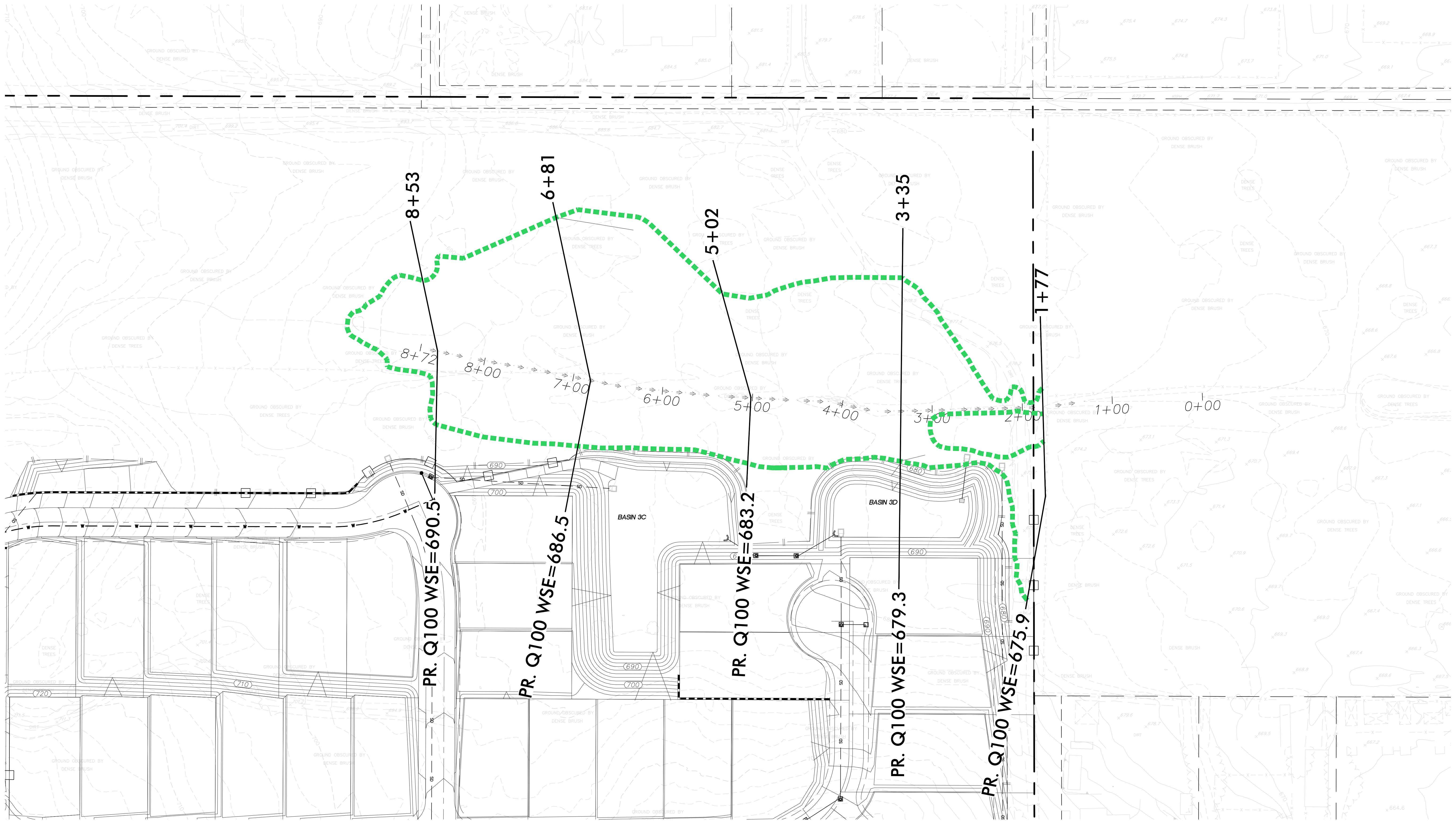
BASIN D
 A=47.0 AC
 Tc=9.8 MIN
 Q=114.85 CFS

BASIN C
 A=173.3 AC
 Tc=12.44 MIN
 Q=340.02 CFS

BASIN E
 A=10.70 AC
 Tc=9.61 MIN
 Q=22.00 CFS

- LEGEND**
- PROJECT BOUNDARY
 - MAJOR BASIN LIMITS
 - SUBBASIN LIMITS
 - FLOW PATH
 - 100 YR INUNDATION LIMITS





LEGEND

- 
 CHANNEL FLOW LINE
- 
 100 YEAR STORM INUNDATION LIMITS (PROPOSED CHANNEL, N = 0.08)